

REPUBLIC OF THE PHILIPPINES DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS

REGION XI DAVAO DEL NORTE 2ND DISTRICT ENGINEERING OFFICE TAGUM CITY

FY 2025 BIP DETAILED ENGINEERING DESIGN PLAN FOR CONSTRUCTION OF WATER SYSTEM IN BARANGAY, MAGWAWA SANTO TOMAS, DAVAO DEL NORTE

PROJECT NAME

MAGWAWA, SANTO TOMAS, DAVAO DEL NORTE LOCATION

SUBMITTED:

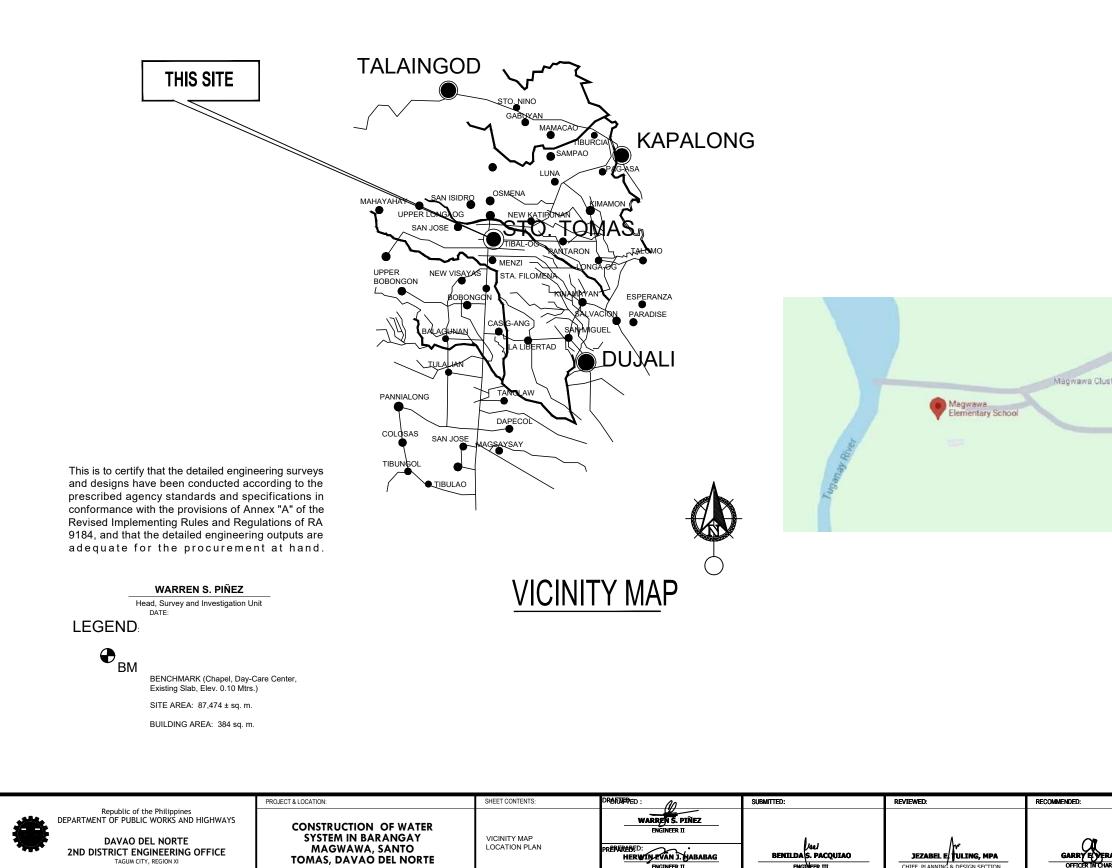
JEZABEL E. TULING, MPA CHIEF, PLANNING AND DESIGN SECTION DATE:

RECOMMENDED:

APPROVED:

GARRY E. VERANO OIC - OFFICE OF THE ASSISTANT DISTRICT ENGINEER DATE:

ARTURO P. LONGYAPON DISTRICT ENGINEER DATE:



ENGINEER III

FICE OF THE ASSISTA

TAGUM CITY, REGION XI

FRANO Harge District Engineer	APPROVED: ARTURO P. LONGYAPON DISTRICT EMSINEER	SET NO:	SHEET NO.:
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		MUNICIPALITY OF S	ITO. TOMAS
		BUILDING O	FFICIAL

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SUMMARY OF QUANTITIES

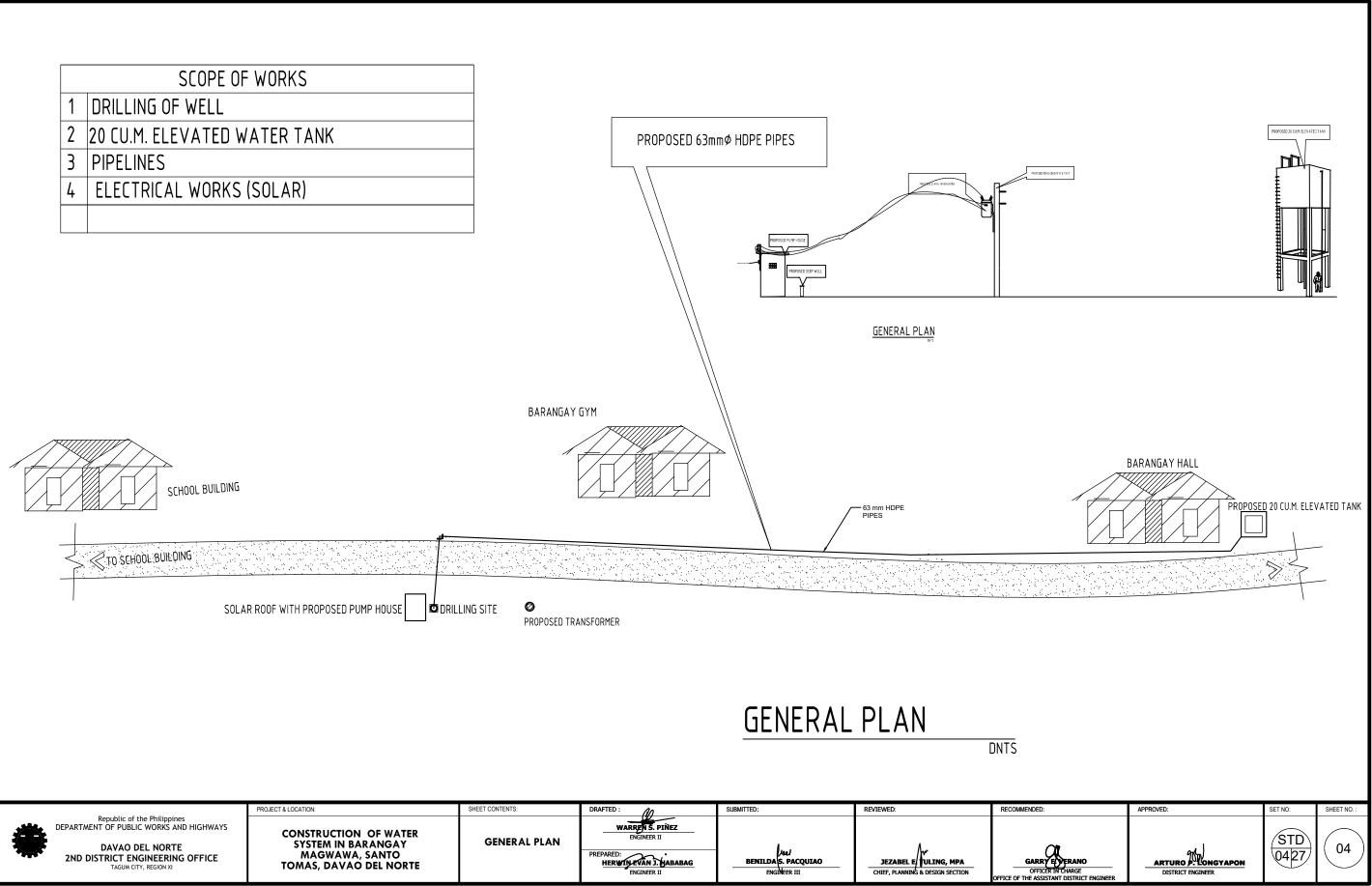
TEM NO.	DESCRIPTION	UNIT	QUANTITY	REMARKS
PART II	OTHER GENERAL REQUIREMENTS			
B.3 (1)	PERMITS AND CLEARANCES	lumpsum	1.00	
B.5 (1)	PROJECT BILLBOARD/SIGN BOARD	each	2.00	
B.7 (1)	OCCUPATIONAL SAFETY AND HEALTH	lumpsum	1.00	
B.9 (1)	MOBILIZATION/DEMOBILIZATION	lumpsum	1.00	
PART C	FINISHING WORKS			
900(1)c	Structural Concrete Class A 28 days (Suspended Slab)20 cu.m. Elevated Tank	cu.m.	11.95	
903(2)	Formworks and Falseworks	sq.m.	60.00	
1002(27)	Plumbing Works	lumpsum	1.00	
1046(2)a1	100mm CHB Non Bearing (including reinforcing steel)	sq.m.	15.60	
1027(1)	Cement Plaster Finish (Pump House with Solar Roof))	sq.m.	41.32	
900(4)c	Structural Concrete (Columns, Beam, footing and tie Beam)20 cu.m. Elevated Tank	cu.m.	10.17	
1032(1)a	Painting Works (Masonry Painting)- for Elevated Tank and Solar Roof with Pump House	cu.m.	123.38	
902(1)a1	Rainforcing Steel (Deformed Grade 40)- 20 cu.m Elevated Tank	kgs.	2,894.68	
1047(8)a	Structural Steel Roof Framing (Pump House with Solar Panel Roof)	kgs.	1,200.00	
1047(3)	Metal Structures Accessories	lumpsum	1.00	
PART E	ELECTRICAL WORKS			
1100(10)	CONDUIT, BOXES AND FITTINGS	lumpsum	1.00	
1101(33)	WIRES AND WIRING DEVICES	lumpsum	1.00	
1102(1)	PANELBOARD WITH MAIN & BRANCH BREAKERS	lumpsum	1.00	
1102(11)	POLE MOUNTED TRANSFORMER WITH COMPLETE ACCESSORIES	lumpsum	1.00	
1102(18)	SOLAR PANEL WITH INVERTER, BATTERY AND OTHER DEVICE	lumpsum	1.00	
1103(1)	LIGHTING FIXTURES AND LAMPS	lumpsum	1.00	
PART I				
1201(1)		lumpsum	1.00	
1600(2)	PIPELINE TRENCH EXCAVATION	cu.m.	126.00	
1602(4)	POLYETHYLENE (PE) PLASTIC PIPE	lumpsum	1.00	
1603(1)	VALVE	lumpsum	1.00	

		PROJECT & LOCATION:	SHEET CONTENTS:	DRORANDED:	SUBMITTED:	REVIEWED:	RECOMMENDED:
۲	Republic of the Philippines DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS DAVAO DEL NORTE 2ND DISTRICT ENGINEERING OFFICE TAGUM CITY, REGION XI	CONSTRUCTION OF WATER SYSTEM IN BARANGAY MAGWAWA, SANTO TOMAS, DAVAO DEL NORTE	SUMMARY OF QUANTITIES	WARREN S. PINEZ ENGINEER II PREPARAMED: HERWIN EVAN J. HABABAG ENGINEER II	BENTILDA S. PACQUIAO ENGIÑEER III	JEZABEL E ULING, MPA CHIEF, PLANNING & DESIGN SECTION	GARRY EVER OFFICE IN CHA

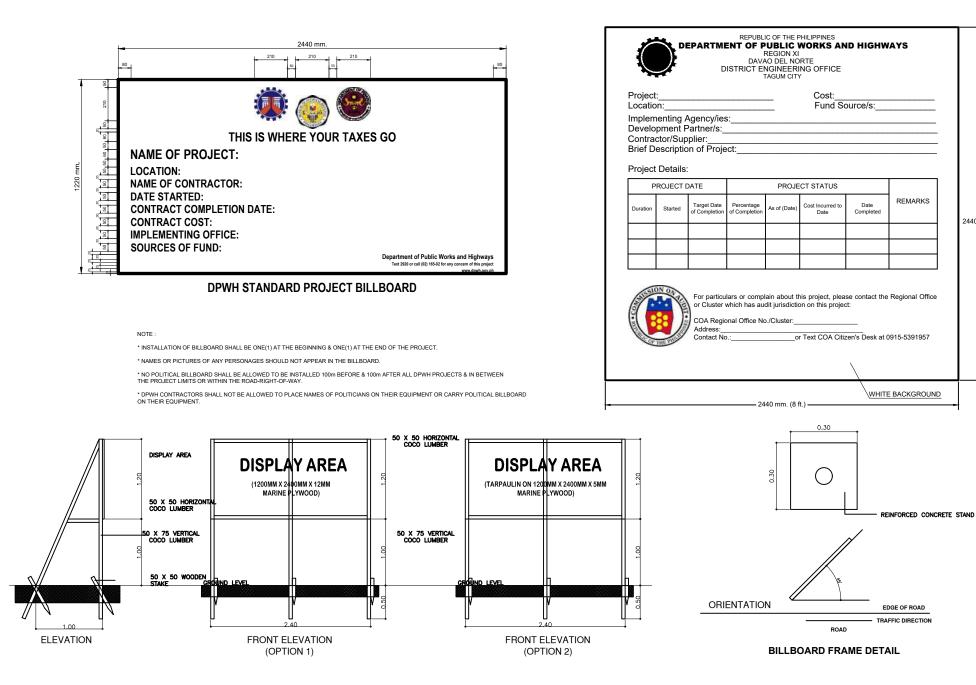
	APPROVED:	SET NO:	SHEET NO. :
FRANO CHARGE DISTRICT ENGINEER	ARTURO P. LONGYAPON DISTRICT ENGINEER	STD 0227	02

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DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS DAVAO DEL NORTE 2ND DISTRICT ENGINEERING OFFICE TAGUM CITY, REGION XI	CONSTRUCTION OF WATER SYSTEM IN BARANGAY MAGWAWA, SANTO TOMAS, DAVAO DEL NORTE	INDEX OF DRAWING	WARREN S. PINEZ ENGINER II PREPARABED: HERWIN EVAN J. MABABAG ENGINEER II	BENTILDA S. PACQUIAO ENGINEER III	JEZABEL FULING, MPA	GARRY EVERAN OFFICE IN CHARGE OFFICE OF THE ASSISTANT DISTRI



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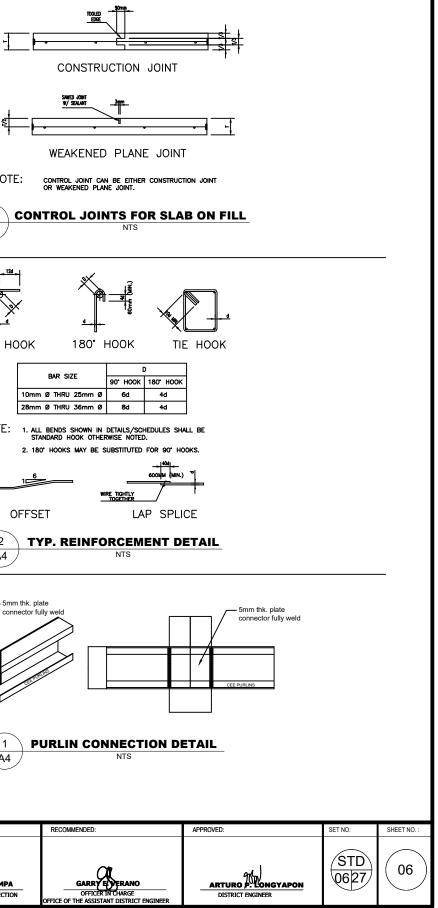


PROJECT & LOCATION: RECOMMENDED: SHEET CONTENTS: REVIEWED: DRAFTED : SUBMITTED: Ŵ Republic of the Philippines DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS WARREN S. PIÑEZ CONSTRUCTION OF WATER IGINEER II DPWH STANDARD / PROJECT SYSTEM IN BARANGAY DAVAO DEL NORTE BILLBOARD/ COA BILLBOARD / FRAMING DETAIL Isel BENILDA S. PACQUIAO MAGWAWA, SANTO EPARED: HERWIN EVAN J. MABABAG 2ND DISTRICT ENGINEERING OFFICE JEZABEL E GARRYE TOMAS, DAVAO DEL NORTE TAGUM CITY, REGION XI OFFICER IN ENGINEER II CHIEF, PLANNING & DESIGN SECTION FICE OF THE ASSISTA

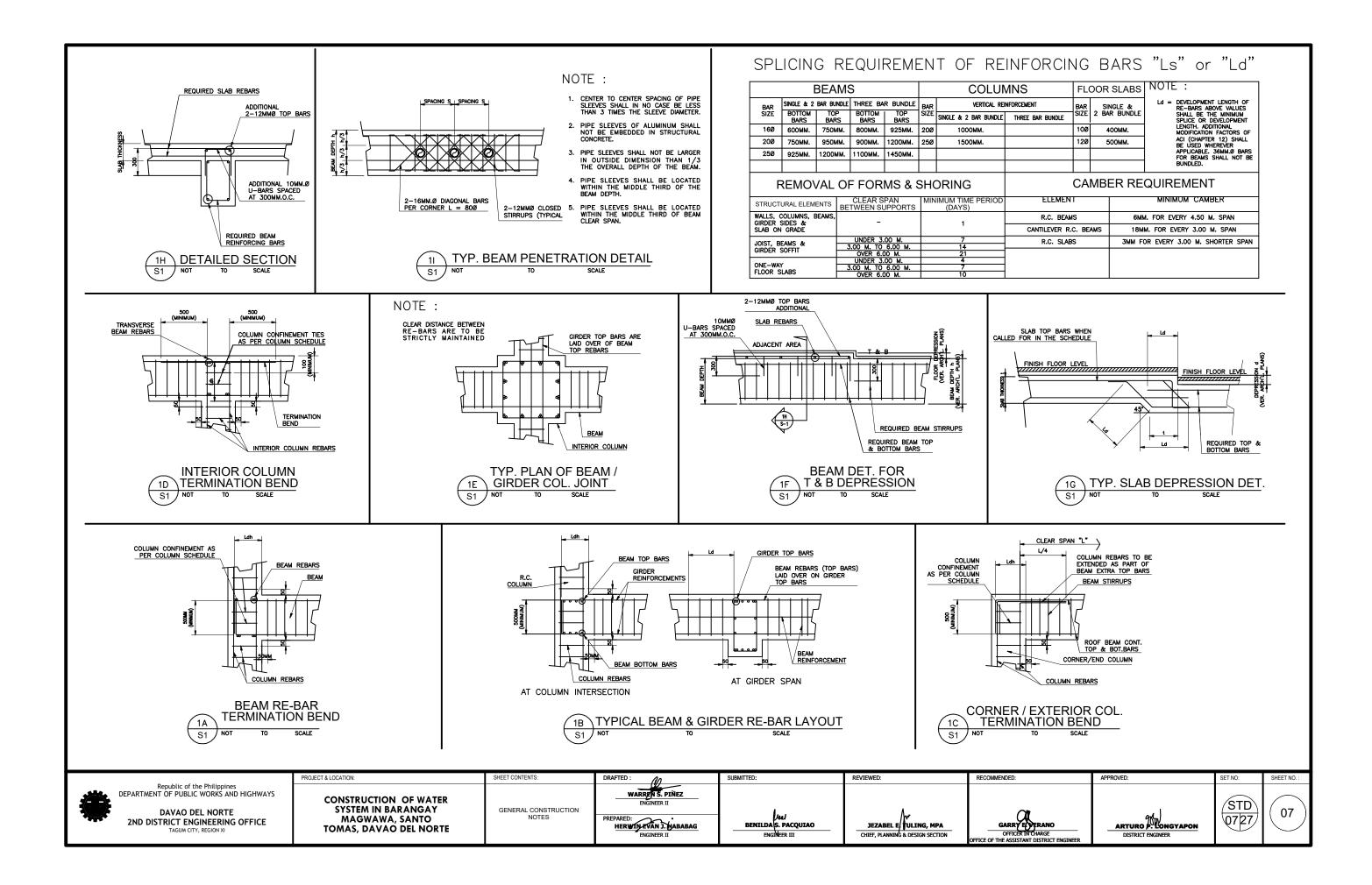
2440 mm. (8 ft.)			
ND			
	APPROVED:	SET NO:	SHEET NO. :
FRANO Harge	ARTURO P. LONGYAPON	STD 0527	05
DISTRICT ENGINEER	DISTRICT ENGINEER		

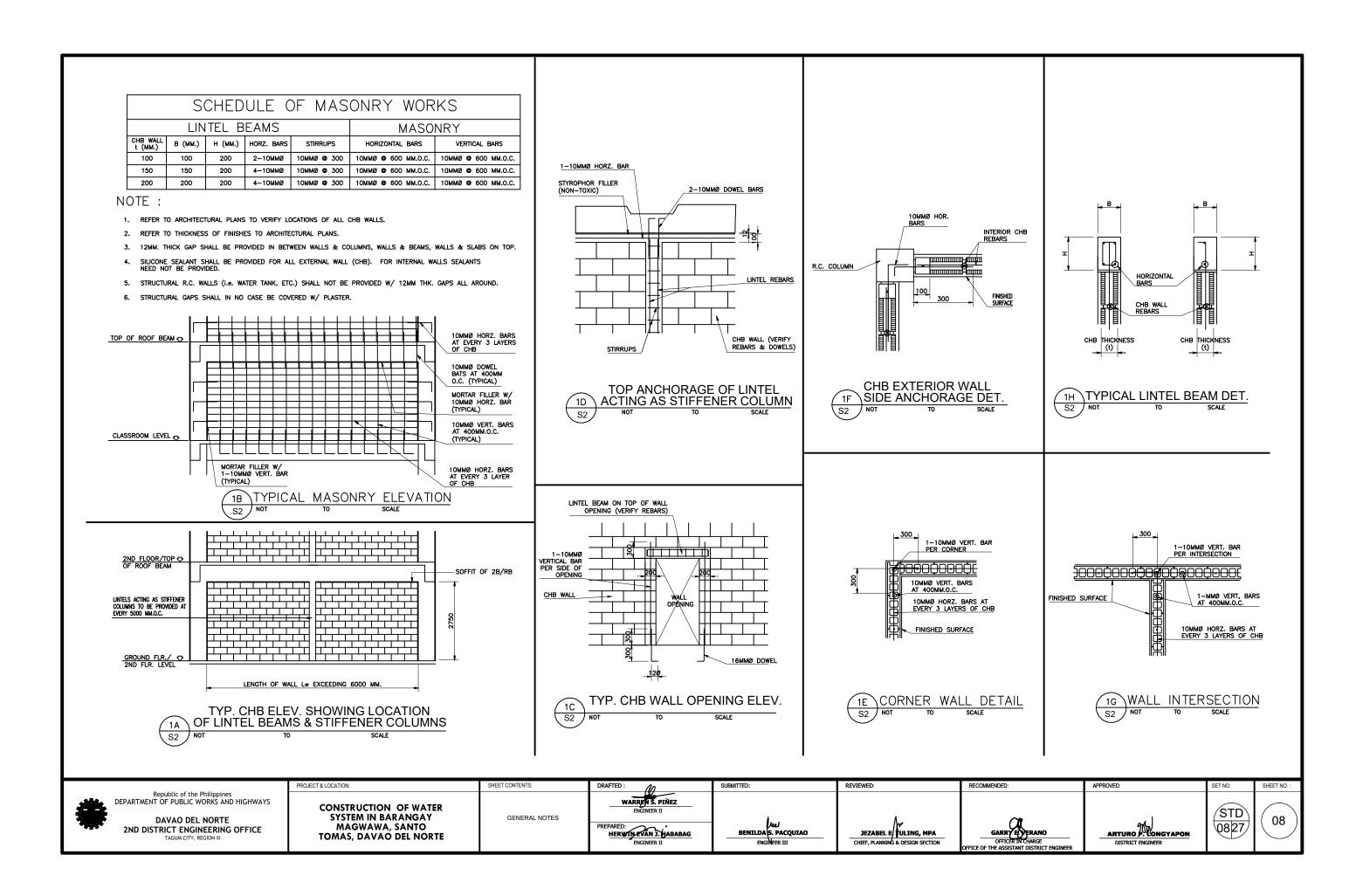
GENERAL NOTES:	4.1 CONCRETE	
1.0 STANDARD & REFERENCES	4.1.1 CONCRETE COVER OVER REINFORCING BARS SHALL BE AS FOLLOWS: A. FODTINGS, FOOTING-TIE BEAMS (CAST AGAINST EARTH) B. BEAMS AND COLUMNS (TO STRUPS AND TES) 40mm	
THE FOLLOWING SHALL GOVERN THE DESIGN, FABRICATION &	B. BEAMS AND COLUMNS (10 SHROP'S AND HES) 40mm C. WALLS, SIDE OF FOOTING-THE BEAMS (CAST AGAINST FORMS) 40mm D. SUSPENDED SLAB 20mm	
CONSTRUCTION OF THE PROJECT	4.1.2 BEFORE CONCRETE IS POURED. CHECK WITH ALL TRADES TO ENSURE PROPER	
1.1 NATIONAL STRUCTURAL CODE OF THE PHILIPPINES (N.S.C.P.) VOL. 7TH. EDITION, 2015	PLACEMENT OF ALL OPENINGS, SLEEVES, CURBS, CONDUITS, ETC. RELATING TO THE WORK.	
2.0 DESIGN CRITERIA	4.2 REINFORCING BARS	
2.1 LOADINGS	4.2.1 ALL REINFORCING BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIALS THAT WILL IMPAIR BOND.	
A. DEAD LOAD	4.2.2 ALL REINFORCING BARS SHALL BE ACCURATELY AND SECURELY PLACED BEFORE POURING CONCRETE OR APPLYING MORTAR OR GROUT.	
CONCRETE - 23.56 Kn/m ³	4.2.3 LAPPED SPLICES SHALL BE STAGGERED WHERE POSSIBLE.	
STEEL - 76.93 Kn/m ³	4.2.4 UNLESS OTHERWISE INDICATED, SPLICING OF REINFORCEMENT SHALL BE IN Accordance with Aci-31BM, Except that the minimum LAP splice shall be	
B. LIVE LOAD ROOF - 1.00 Kn/m ³	40 BAR DIAMETER BUT NOT LESS THAN 600mm.	
C. WIND LOAD (NSCP 2010)	4.2.5 UNLESS SHOWN OTHERWISE ON PLANS, SPLICES SHALL BE AS FOLLOWS:	
BASIC WIND VELOCITY, V = 270 KPH P=QH [9GCpf]] (DESIGN WIND PRESSURE)	A. INTERMEDIATE BEAMS: TOP BARS SHALL BE SPLICED AT MID-SPAN, AND BOTTOM BARS AT THE SUPPORT.	3
where: qh = VELOCITY PRESSURE, Kpa	B. BEAMS FRAMING TO COLUMNS: TOP BARS SHALL BE SPLICED AT MID-SPAN AND BOTTOM BARS SHALL NOT BE SPLICED W/IN THE COLUMN OR W/IN A DISTANCE OF TWICE THE MEMBER DEPTH FROM THE FACE OF THE COLUMN. THE SPLICED	
GCpf = EXTERNAL PRESSURE COEFFICIENT GCpi = INTERNAL PRESSURE COEFFICIENT	LENGTH SHALL NOT BE LESS THAN 1.4 TIMES THE DEVELOPMENT LENGTH (Ld) IN 4.2.8 BELOW BUT NOT LESS THAN THAN 600mm.	
D. SEISMIC LOAD (NSCP 2010)	COUNT OF THE WEEKEN OF THE TABLE OF THE OFFICE OF THE OFFICE OFFI	
V= <u>CVL</u> (W) (DESIGN BASE SHEAR) RT	THE USE OF APPROVED MECHANICAL DEVICES MAY BE PERMITED PROVIDED Not more than alternate bars are welded or spliced at any level and the minimum vertical distance between two adjacent bar splices shall	
Vmax = 2 <u>.50 cal</u> (W) Vmin = 0.11 CalW	D. CHB WALLS: VERTICAL BARS SHALL BE SPLICED AT THE TOP OF WALL FOOTINGS OR FOOTING-THE BEAMS AND AT THE BOTTOM OF REINFORCED CONCRETE LINTEL	
Vmin = 0.80 <u>ZNvI</u> W (ZONE 4) R	OR FOOTING-TIE BEAMS AND AT THE BOTTOM OF REINFORCED CONCRETE LINTEL BEAMS OR BEAMS.	
WHERE: W = TOTAL DEAD LOAD	4.2.6 UNLESS OTHERWISE INDICATED: ALL BEAMS TERMINATING AT A COLUMN SHALL HAVE TOP AND BOTTOM BARS EXTENDING TO THE FAR FACE OF THE COLUMN, TERMINATING	
T = NATURAL PERIOD = Ct (hn)% WHERE: C = NUMERICAL COEFFICIENT	IN A STANDARD 90 HOOK LENGTH OF ANCHORAGE SHALL NOT BE LESS THAN 600mm.	-
h = BUILDING HEIGHT	4.2.7 SHOP DRAWING FOR REINFORCEMENT SHALL BE SUBMITTED FOR APPROVAL OF THE ENGINEER PRIOR TO FABRICATION & INSTALLATION.	90
I = IMPORTANCE FACTOR = 8.50 SEISMIC COEFFICIENT Cv = 0.44 Nv	4.2.8 DEVELOPMENT LENGTH (Ld) OF REINFORCING BARS SHALL BE AS FOLLOWS: SIZE OF REBARS DEVELOPMENT LENGTH	
Ca = 0.64N	10 mm 170 mm 12 mm 220 mm	
NEAR SOURCE FACTOR (10 Km) Nv = 1.2 Na = 1.0	16 mm 270 mm 20 mm 380 mm 25 mm 600 mm	
Z = SEISMIC ZONE = 0.40 (ZONE 4)	4.3 STRUCTURAL STEEL	
S = SOIL TYPE = D	4.3.1 ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 AND SHALL HAVE A MINIMUM	
2.1 DESIGN STRESSES	YIELD STRESS, Fy = 248 MPg (36,000 pgi) 4.3.2 All structural steel shall be fabricated and erected in accordance with	NO
A. CONCRETE	THE AISC SPECIFICATIONS AND CODE OF STANDARD PRACTICE AS AMMENDED TO DATE.	
COMPRESSIVE STRENGTH (@ 28 DAYS fc' = 27 MPa (4,000 psi)	4.3.3 ALL BOLTS SHALL CONFORM TO ASTM A-307 UNLESS OTHERWISE INDICATED. 4.3.4 SHOP AND FIELD WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 AND PERFORMED	
fc' = 21 MPa (3,000 psi) B. REINFORCING BARS	BY QUALIFIED WELDERS. 4.3.5 UNLESS OTHERWISE INDICATED, WELDING ELECTRODES SHALL BE E60.	
a. FOR BARS 16 MM Ø & GREATER fy = 420 MPA (60,000 PSI)	4.3.6 NO STEEL SHALL BE FABRICATED OR ERECTED UNTIL SHOP DRAWINGS HAVE BEEN APPROVED BY THE STRUCTURAL ENGINEER.	
b. FOR BARS LESS THAN 16 mm ϕ fy = 275 MPA (40,000 PSI) C. STRUCTURAL STEEL, ASTM-A36	4.3.7 WELDS/(CONFORM WITH AMERICAN WELDING STANDARDS) USING E $60xx$ electodes. fy = 93.77 MPa.	
3.0 FOUNDATION	4.3.8 ANCHOR BOLTS (CONFORM WITH ASTM A-307) ft = 96.60 MPa. fv = 69 MPa.	
3.1 FOUNDATIONS ARE DESIGNED USING AN ASSUMED ALLOWABLE SOIL BEARING CAPACITY OF	 4.4 CONCRETE HOLLOW BLOCKS (CHB): 4.4.1 UNLESS OTHERWISE INDICATED, CHB USED IN THIS WORK SHALL HAVE A MINIMUM 	(
	ULTIMATE COMPRESSIVE STRENGTH f'm = 3.45 MPa (500 psi)	(
3.1.1 IN CASE THE ACTUAL LOCATION OF THE STRUCTURE IS LESS THAN THE ASSUMED	4.4.2 ALL CHB CELLS SHALL BE FILLED SOLIDLY WITH GROUT.	
DISTANCE FROM THE SEISMIC SOURCE OF 40km; NOTIFY THE DIRECTOR, BUREAU OF DESIGN FOR PROPER REVISION OF THE DESIGN. REFER TO THE SEISMIC	5.0 CONSTRUCTION JOINT	
SOURCE MAP PROVIDED IN THE NATIONAL STRUCTURAL CODE OF THE PHILIPPINES OR PHIVOLCS SEISMIC SOURCE MAP.	5.1 CONSTRUCTION JOINT NOT INDICATED ON THE PLANS SHALL BE MADE SO AS TO LEAST IMPAIR THE STRENGTH OF THE STRUCTURE AND SHALL BE SUBJECT TO THE APPROVAL	
3.1.2 SOIL TEST SHALL BE CONDUCTED PRIOR TO START OF CONSTRUCTION. 3.1.3 IN CASE THE ACTUAL SOIL BEARING CAPACITY IS FOUND LESS THAN THE ASSUMED,	OF THE ENGINEER EXCEPT SLAB ON GRADE.	Γ
96 kPa; NOTIFY THE DIRECTOR, BUREAU OF DESIGN FOR PROPER REVISION OF FOUNDATION.	5.2 UNLESS SHOWN OTHERWISE, SLAB ON GRADE SHALL HAVE CONTROL JOINTS SPACED AT 6000mm MAXIMUM CENTER TO CENTER.	
3.1.4 NO FOOTING SHALL REST ON FILL. 3.1.5 BOTTOM OF FOOTING SHALL BE AT LEAST 1.00m. BELOW NATURAL GRADE LINE.	5.3 BEAMS CONSTRUCTION JOINT SHALL BE LOCATED W/ IN THE MIDDLE THIRD OF THE SPAN. IT SHALL BE PROVIDED WITH 3 EXTRA STIRRUPS ● 75mm O.C. ON EACH SIDE OF THE	
3.1.6 SOIL BEARING CAPACITY SHALL BE INCREASED BY 33% WHEN IN COMBINATION WITH	JOINT.	
SEISMIC OR WIND LOAD.	REFERENCES :	
3.2 ALL COLUMN FOOTINGS & TIE BEAMS SHALL REST ON 100mm THK. WELL COMPACTED GRAVEL BASE COURSE.	 Labor Code of the Philippines and its Implementing Rules and Regulations DOLE DO No. 13, s. 1999, Occupational Safety and Health Standards and its 	
3.3 BACK FILL SHALL BE PLACED IN LAYER AND EACH LAYER SHALL BE 200mm THK. AND SHALL BE COMPACTED TO 95% MAXIMUM DRY DENSITY.	DULE DU No. 13, s. 1996, Occupational Safety and Health Standards and its Procedural Guidelines. - For monitoring, enforcement and implementation of construction safety and health	
SHALL BE COMPACIED TO 95% MAXIMUM DRT DENSITY. 3.4 WHERE LOOSE/SOFT MATERIAL IS ENCOUNTERED AT DEPTH OF EMBEDMENT INDICATED,	- DO . 56, s. 2005	
EXCAVATE TO FIRM LAYER AND REPLACE LOOSE/MATERIALS UNDERNEATH THE FOOTING	 DPWH Design Guidelines, Criteria and Standards (DGCS), 2015 Edition For the design of highways, bridges, buildings and flood control projects covering 	
WITHIN THE FOOTING AREA PLUS 1/2 DEPTH OF SOFT MATERIAL ON ALL SIDES WITH SELECT GRANULAR BACKFILL. COMPACT SELECT GRANULAR BACKFILL TO 95% OF MAXIMUM DRY DENSITY.	the minimum requirements, specifications and procedures. - D0. 179, s. 2015	
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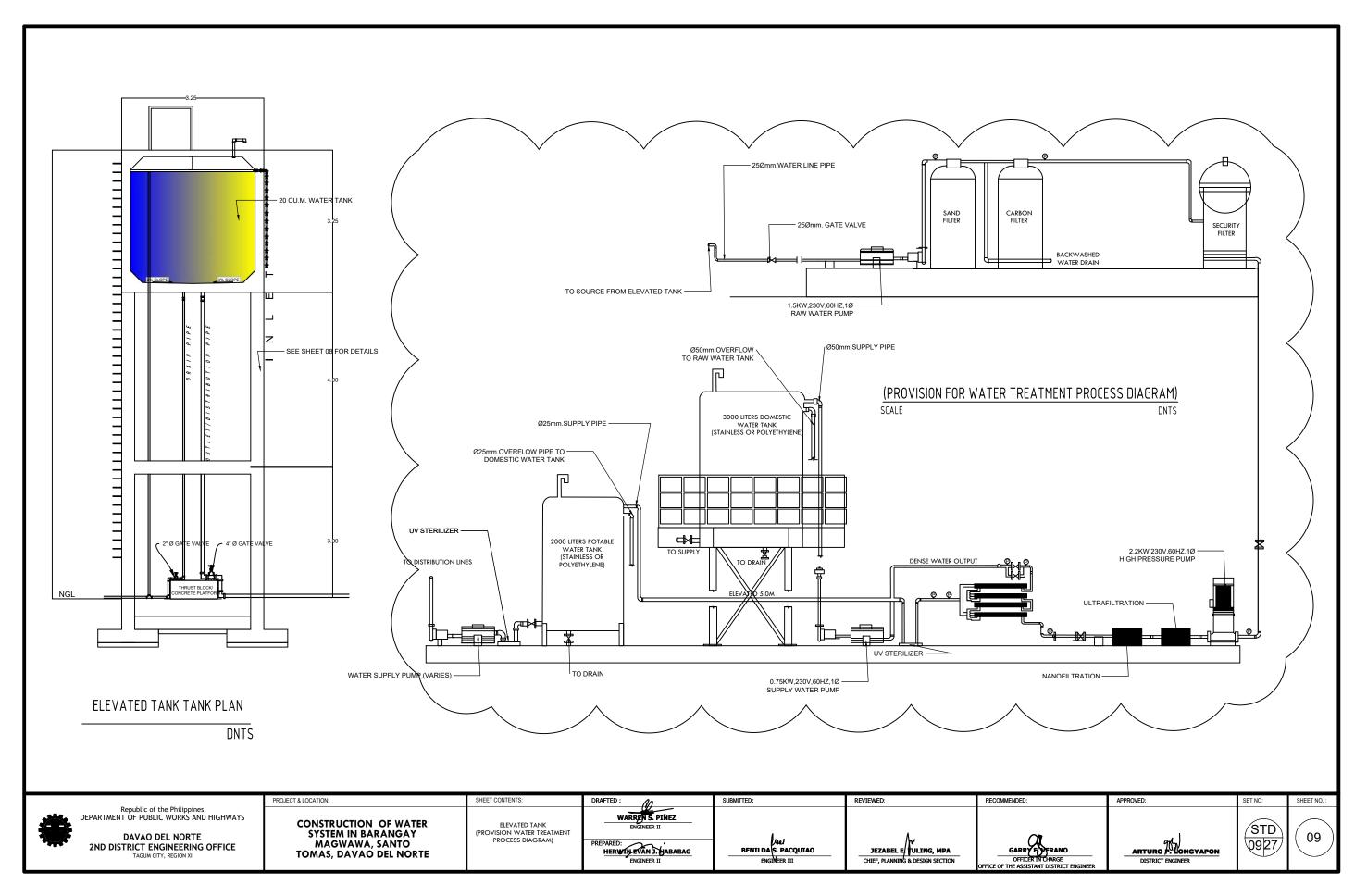
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BAR SIZE







	BEAMS LEVEL 1 & FOOTING TIE BEAMS							COL	UMNS		F	οτιν	GS			
CAPACITY (m³)	DIME	NSIONS (mm)	REINFO	RCEMEN	IT (mm)	SECT	TIONS		REBARS		SECTION		BP 120 K		REMARKS
	с	D	E	тв	вв	STIRR.	SUPPORT	MIDSPAN	(mm)	1m) (mm)	(mm)		W (mm)	tf (mm)	REBARS (mm) BOTHWAYS	
20	300	300	3250	2-D16	2-D16	D10 1 50 fr ends rest 150			300x300	12-D16	D10 @ 250 2/SET	\bigcirc	1520x1620	300	10-D12	h1 = 3000 h2 = 4000

SCHEDULE OF DIMENSION AND REINFORCEMENT OF ELEVATED WATER TANK

	WATER TANK						BEAMS @ LEVEL 2																					
CAPACITY	DI	IMENSIO	NS (mm)				REI	NFORCE	MENT (r	nm)				DIME	NSION	(mm)									SECTIO	NS (mm))
(m³)	а	b1	b2	е	1 bw	2 bw	3 bw	4 bw	5	6	7	8 bw	9 bw	10 bw	c	d	е	1	2	3	4	5	6	stirrups	A	в	с	D
20	2850	200	150	3250	D12 @ 104	D12 @ 208	D12 @ 104	D12 @ 208	D12 @ 104	D12 @ 208	D12 @ 208	D12 @ 300	D12 @ 300	D12 @ 300	200	600	3250	2-D16	2-D16	2-D16	4-D20	2-D16	2-D20	D10 1@50 fr. ENDS REST @ 250				

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	APPROVED:	SET NO:	SHEET NO. :
VERANO CHARGE DISTRICT ENGINEER	APPROVED:	SET NO:	SHEET NO.:

SCHEDULE OF REBAR SPLICES AND LENGTH OF EMBEDMENT

BAR SIZE	LENGTH OF LAPPED SPLIC	ES FOR REINFORCING (mm)	LENGTH OF EMBEDMENT FOR END ANCHORAGE OF REINFORCING w/ STANDARD HOOKS (mm)			
	* TOP BARS	OTHERS	* TOP BARS	OTHERS		
D 10	500	350	500	350		
D 12	575	500	400	450		
D 16	725	500	500	450		
D 20	925	650	650	500		
D 22	1250	900	900	550		
D 25	1650	1175	1100	650		
D 28	2075	1500	1375	800		
D 32	2650	1900	1650	1025		
D 36	3250	2325	1950	1300		

MINIMUM CONCRETE COVER FOR REINFORCEMENT

UNFORMED SURFACES ADJACENT TO EXCAVATION

BARS IN BEAM OR GIRDER, INCLUDING STIRRUPS

LOCATION

FORMED OR TOP SURFACES EXPOSED TO WEATHER OR SATURATED AIR, SUBMEREGED

D 20 or LARGER BARS

D 20 or LARGER BARS

AND COLUMN SPIRAL OR TIES

LARGER THAN D 36 BARS D 36 AND SMALLER BARS

OR IN CONTACT WITH EARTH

OTHER LOCATION:

MINIMUMCOVER

75

40

40

40 20

* TOP BARS ARE HORIZONTAL BAR SO PLACED THAT >300 OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR. HORIZONTAL BARS IN WALLS ARE TO PROVIDED WITH AS REQUIRED FOR TOP BARS. EXCEPT AS OTHERWISE INDICATED ON THE PLANS, EMBEDMENT LENGTHS FOR END ANCHORAGE AND LAPPED SPLICES SHALL NOT BE LESS THAN (NO MINUS TOLERANCE)SHOWN ABOVE.

LAPPED SPLICES SHALL NOT BE MADE AT POINT OF MAXIMUM STRESS DETERMINED BY THE ENGINEER, AND SHALL NOT BE SPACED CLOSER THAN 150 ON CENTERS. IF SPLICES ARE STAGGERED SO THAT NO MORE THAN $\frac{1}{2}$ ARE SPLICED ON A LAP SPLICE LENGTH, THE SPLICE LENGTH CAN BE REDUCED TO 75 % OF THE LENGTH TABULATED ABOVE.

GENERAL NOTES: 50

1. ALL DIMENSION ARE IN MILLIMETER UNLESS OTHERWISE SPECIFIED.
2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH, fc'=20.69 Mpa EXCEPT LEVELING CONCRETE
WHICH SHALL RE 12 80 Mpg AT 29 DAVS BASED ON STANDARD OVI INDER

- WHICH SHALL BE 13:80 Mpa AT 28 DAYS BASED ON STANDARD CYLINDER. 3. REINFORCEMENT BARS SHALL BE DEFORMED BILLET STEEL BARS, INTERMEDIATE GRADE WITH MINIMUM YIELD STRENCTH, fy, OF 276 Mpa AND ALLOWABLE TENSILE STRENGTH OF 138 Mpa CONFORMING TO ASTM A-615. 4. ALLOWABLE SOIL BEARING PRESSURE=120 Kpa
 - 5. ABBREVIATIONS:

 - TB -BB -BW -
 - TOP BARS BOTTOM BARS BOTHWAYS INSIDE DIMENSIONS OF TANK
 - a -b -

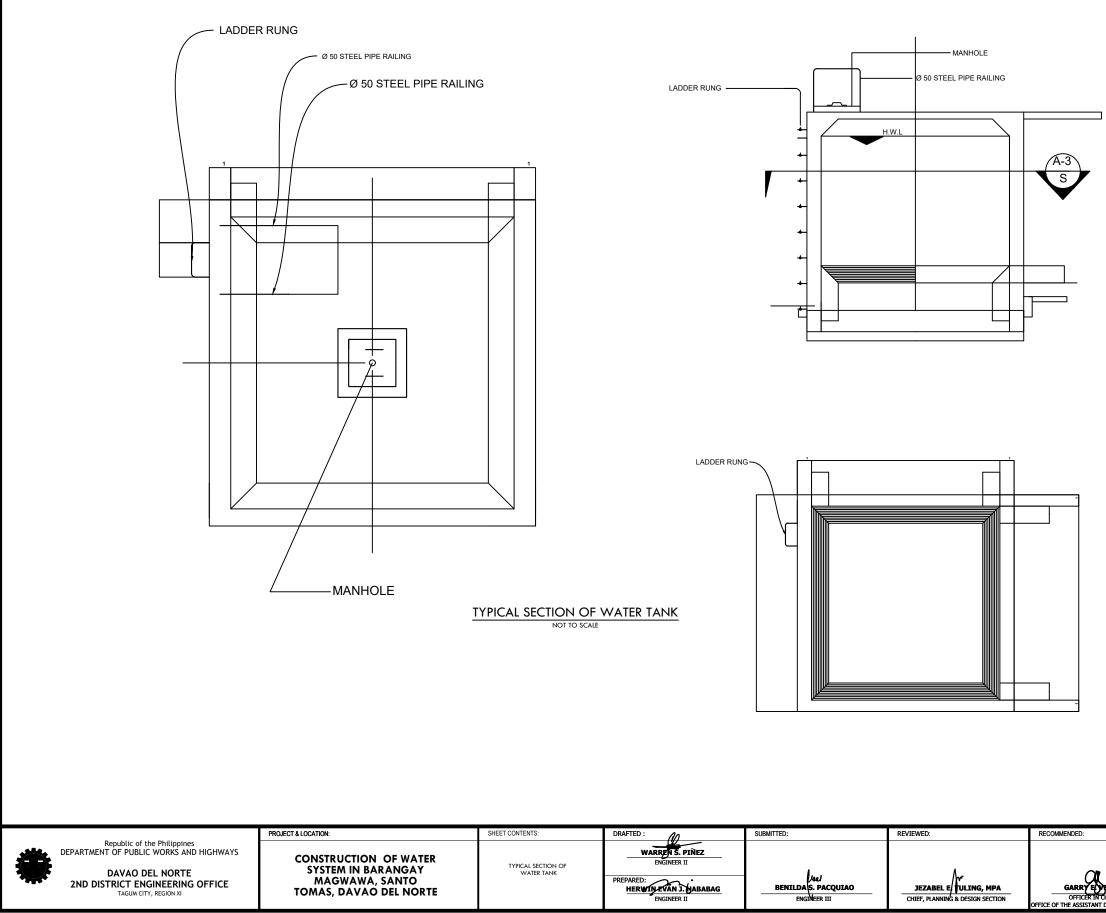
 - c -d -
 - INSIDE DIMENSIONS OF TANK THICKNESS OF WALLS THICKNESS OF TOP SLAB THICKNESS OF BOTTOM SLAB OUTSIDE DIMENSION OF TANK MAXIMUM HIGH WATER LEVEL THICK EACH FACE e -MAX -

 - H.W.L. -THK -EF -

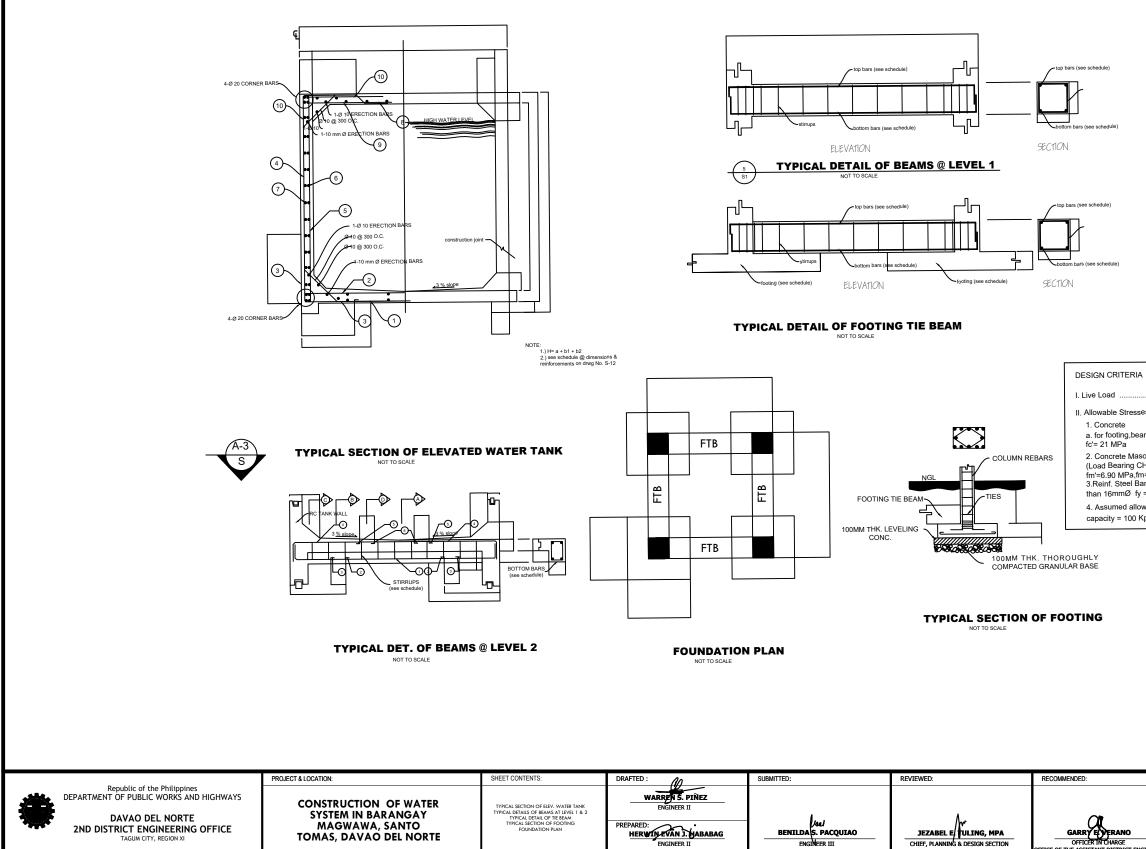
 - `₩ WELD

	PROJECT & LOCATION:	SHEET CONTENTS:	DRAFTED :	SUBMITTED:	REVIEWED:	RECOMMENDED:
DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS DAVAO DEL NORTE DISTRICT ENGINEERING OFFICE TAGUM CITY, REGION XI	CONSTRUCTION OF WATER SYSTEM IN BARANGAY MAGWAWA, SANTO TOMAS, DAVAO DEL NORTE	MINIMUM COVER OF REINFORCEMENT SCHEDULE OF REAR SPLICES AND EMBEDMENT	PREPARED: HERWIN EVAN J. HABABAG ENGINEER II	BENILDA S. PACQUIAO ENGINEER III	JEZABEL E TULING, MPA CHIEF, PLANNING & DESIGN SECTION	GARRY EDER OFFICER IN CHAR OFFICE OF THE ASSISTANT DIS

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. 2000 Pa II. Allowable Stresses

a. for footing,beams and slabs fc'= 21 MPa

2. Concrete Masonry Units

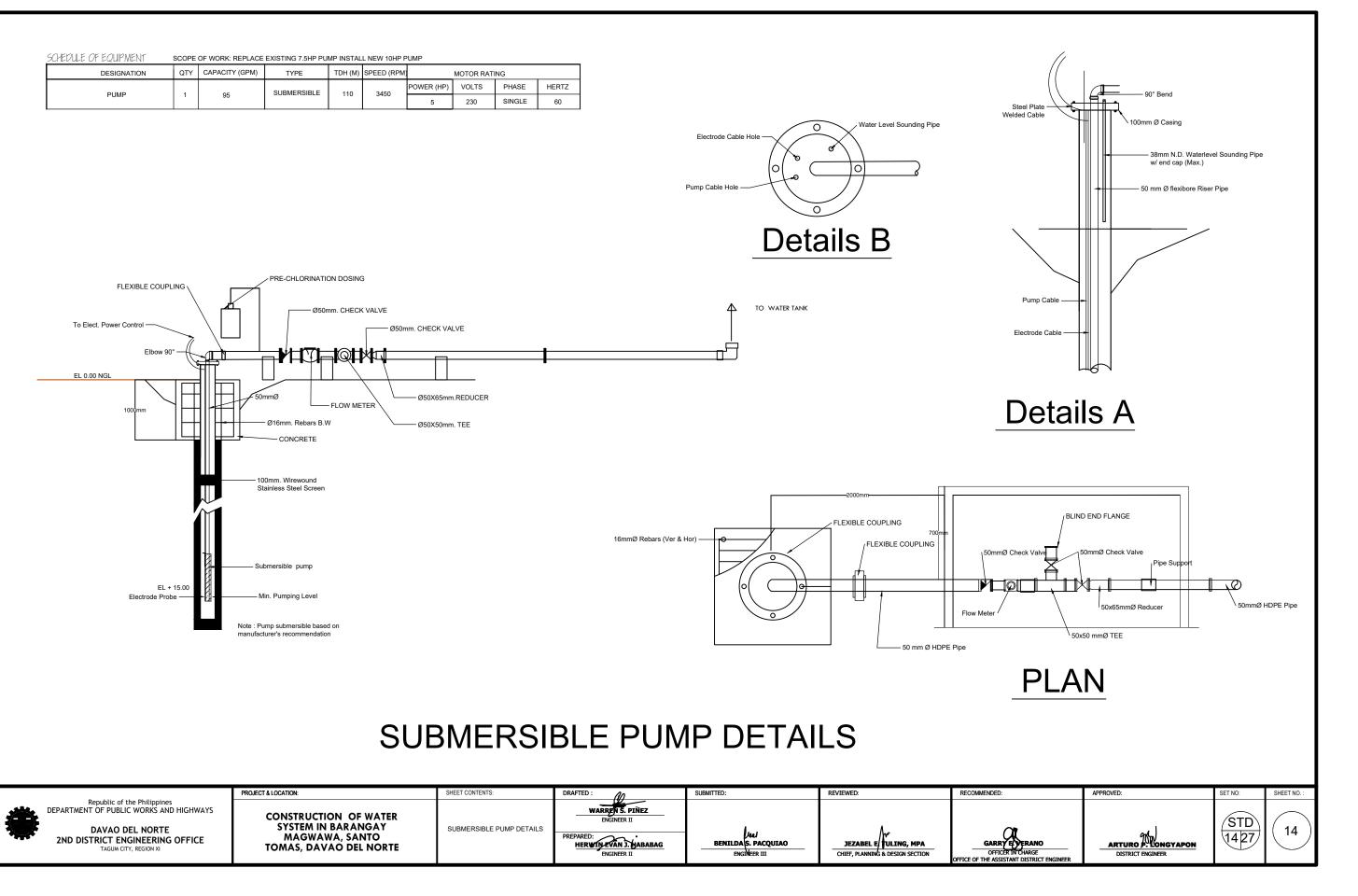
2. Concrete Masoriny Onlis (Load Bearing CHB) fm'=6.90 MPa,fm= 2.41 MPa 3.Reinf. Steel Bars for bars smaller than 16mmØ fy = 230 MPa

4. Assumed allowable soil bearing

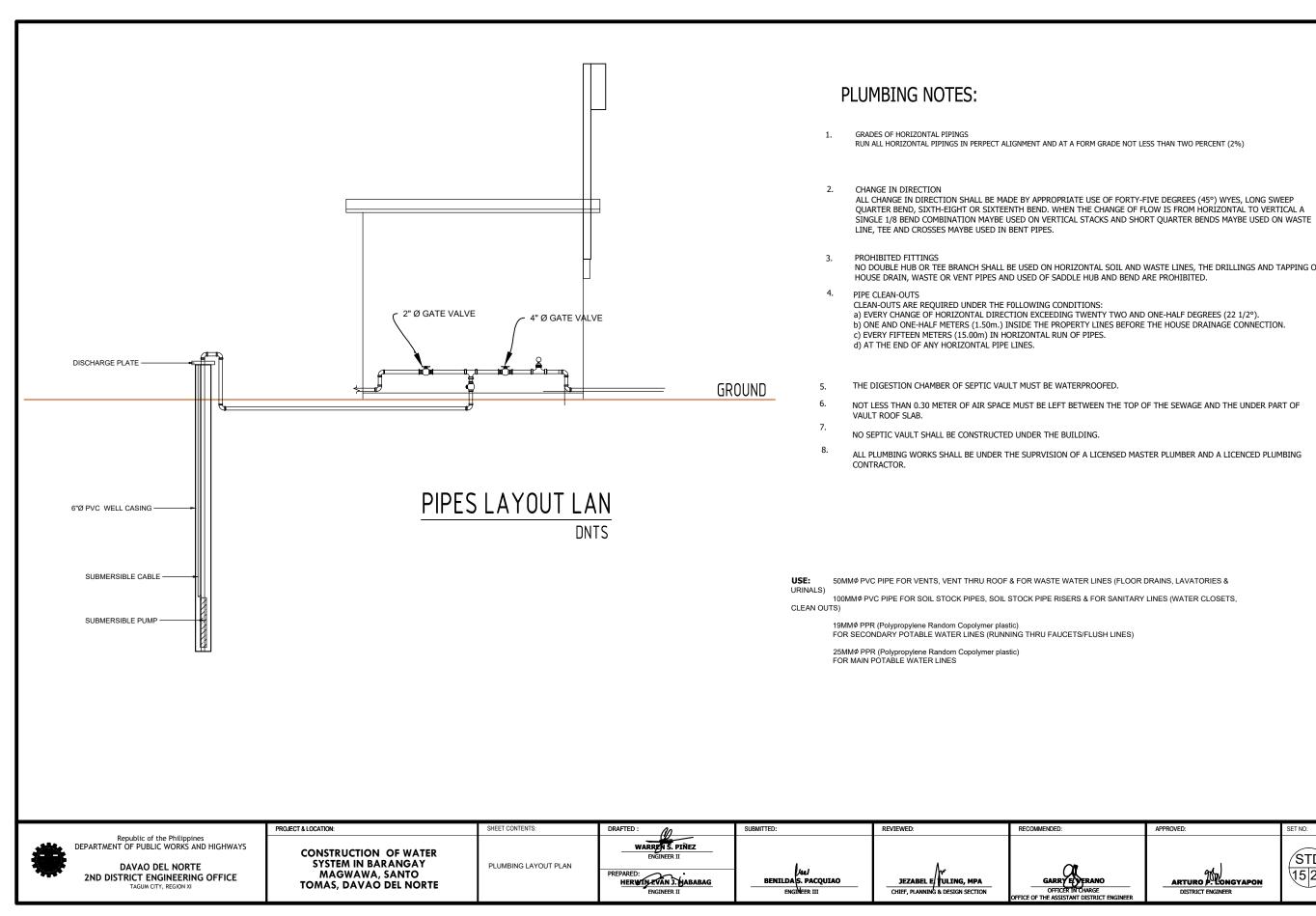
capacity = 100 Kpa

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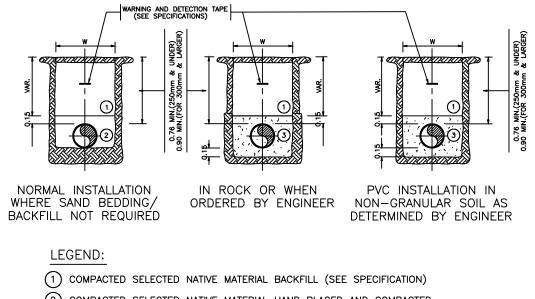
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Republic of the Philippines DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS DAVAO DEL NORTE 2ND DISTRICT ENGINEERING OFFICE TAGUM CITY, REGION XI	CONSTRUCTION OF WATER SYSTEM IN BARANGAY MAGWAWA, SANTO TOMAS, DAVAO DEL NORTE	SUBMERSIBLE PUMP DETAILS	WARREN S. PIÑEZ ENGINEER II PREPARED: HERVIN EVAN J. MABABAG ENGINEER II	BENILDA S. PACQUIAO ENGIÑEER III	JEZABEL E ULING, MPA CHIEF, PLANNING & DESIGN SECTION	GARRY EVERA OFFICER IN CHAR OFFICE OF THE ASSISTANT DISTI



NO DOUBLE HUB OR TEE BRANCH SHALL BE USED ON HORIZONTAL SOIL AND WASTE LINES, THE DRILLINGS AND TAPPING OF APPROVED: SET NO: SHEET NO. STD 15 15 27 ARTURO P. LONGYAPON



(2) COMPACTED SELECTED NATIVE MATERIAL HAND PLACED AND COMPACTED IN 0.15M LAYER (SEE SPECIFICATION)

(3) APPROVED SAND BEDDING AND BACKFILL HAND PLACED AND COMPACTED

TABLE OF TRENCH DIMENSION (IN METRES)	TABLE O)F TRENCH	DIMENSION	(IN	METRES)
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PIPE	e diameter	mm	50	63	75	110	160	300	350	400	450	500	600	700	800	900	1000	1100
МІМ	NIMUM "W"	m	0.20	0.30	0.30	0.30	0.30	0.60	0.65	0.70	0.75	0.80	0.90	1.00	1.10	1.20	1.30	1.40
MA	XIMUM "W"	m	0.30	0.40	0.50	0.60	0.60	0.90	0.95	1.00	1.05	1.10	1.20	1.30	1.40	1.50	1.60	1.70

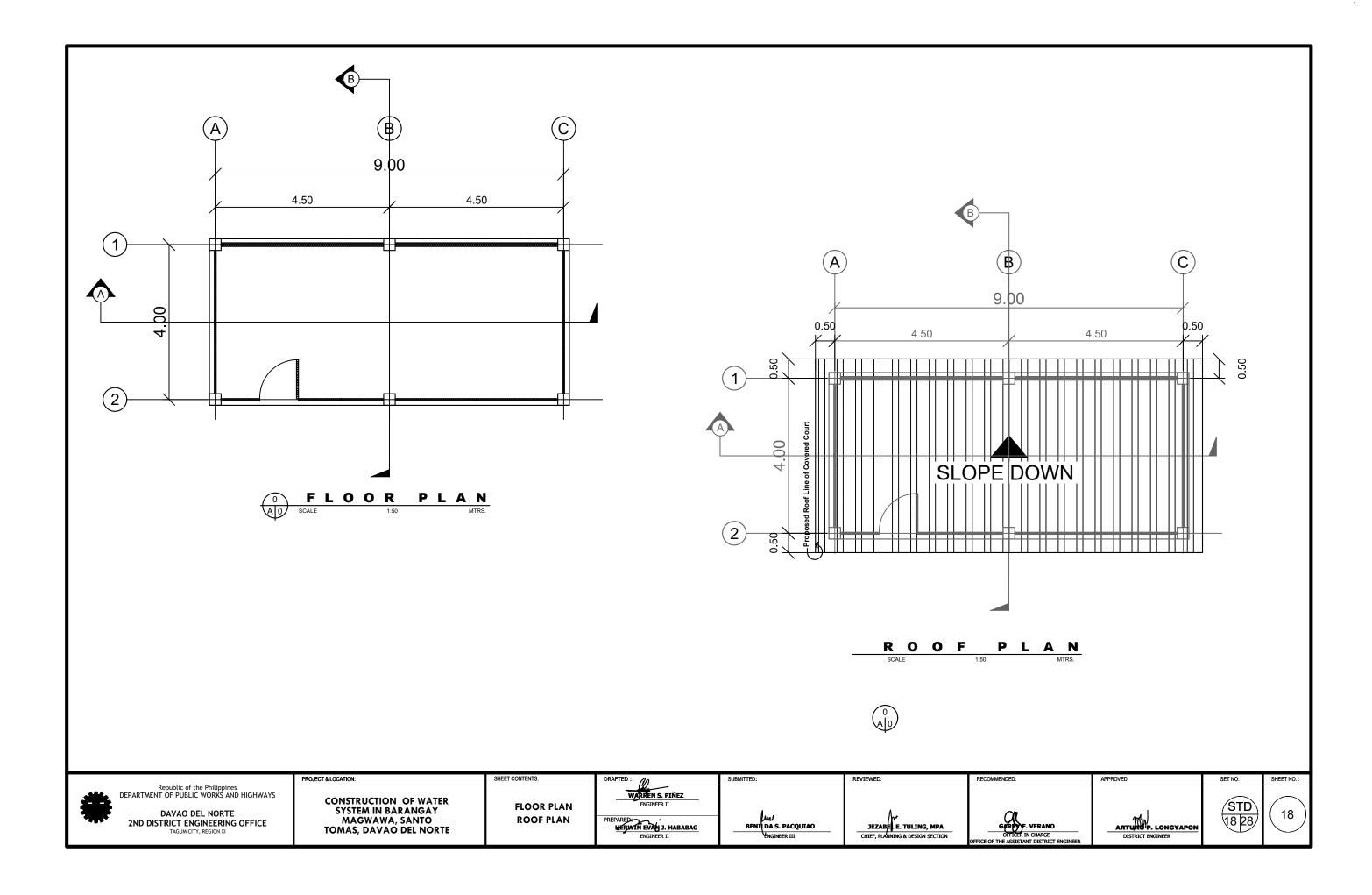
TYPICAL TRENCH DETAILS

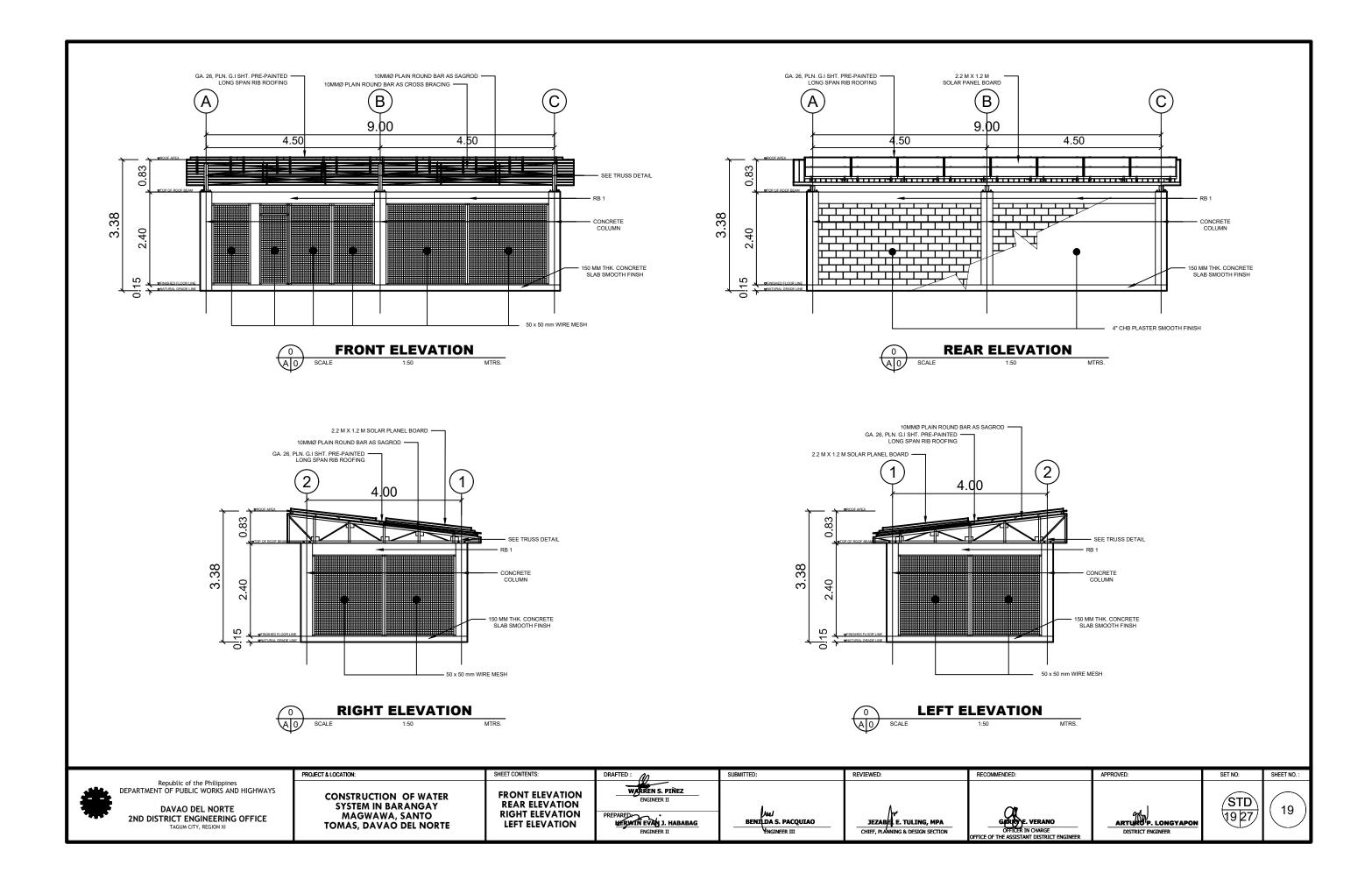
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		PROJECT & LOCATION:	SHEET CONTENTS:	DRAFTED :	SUBMITTED:	REVIEWED:	RECOMMENDED:
	Republic of the Philippines DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS DAVAO DEL NORTE 2ND DISTRICT ENGINEERING OFFICE TAGUM CITY, REGION XI	CONSTRUCTION OF WATER SYSTEM IN BARANGAY MAGWAWA, SANTO TOMAS, DAVAO DEL NORTE	TYPICAL TRENCH DETAILS	WARNEN PINEZ ENGINER II PREPARED: HERWIN EVAN 3. HABABAG ENGINEER IP	BENILDA (MACQUIAO ENGIVEER III	JEZABEL E ULING, MPA CHIEF, PLANNING & DESIGN SECTION	GARRY FEREN OFFICED ACHAR OFFICE OF THE ASSISTIANT DIST

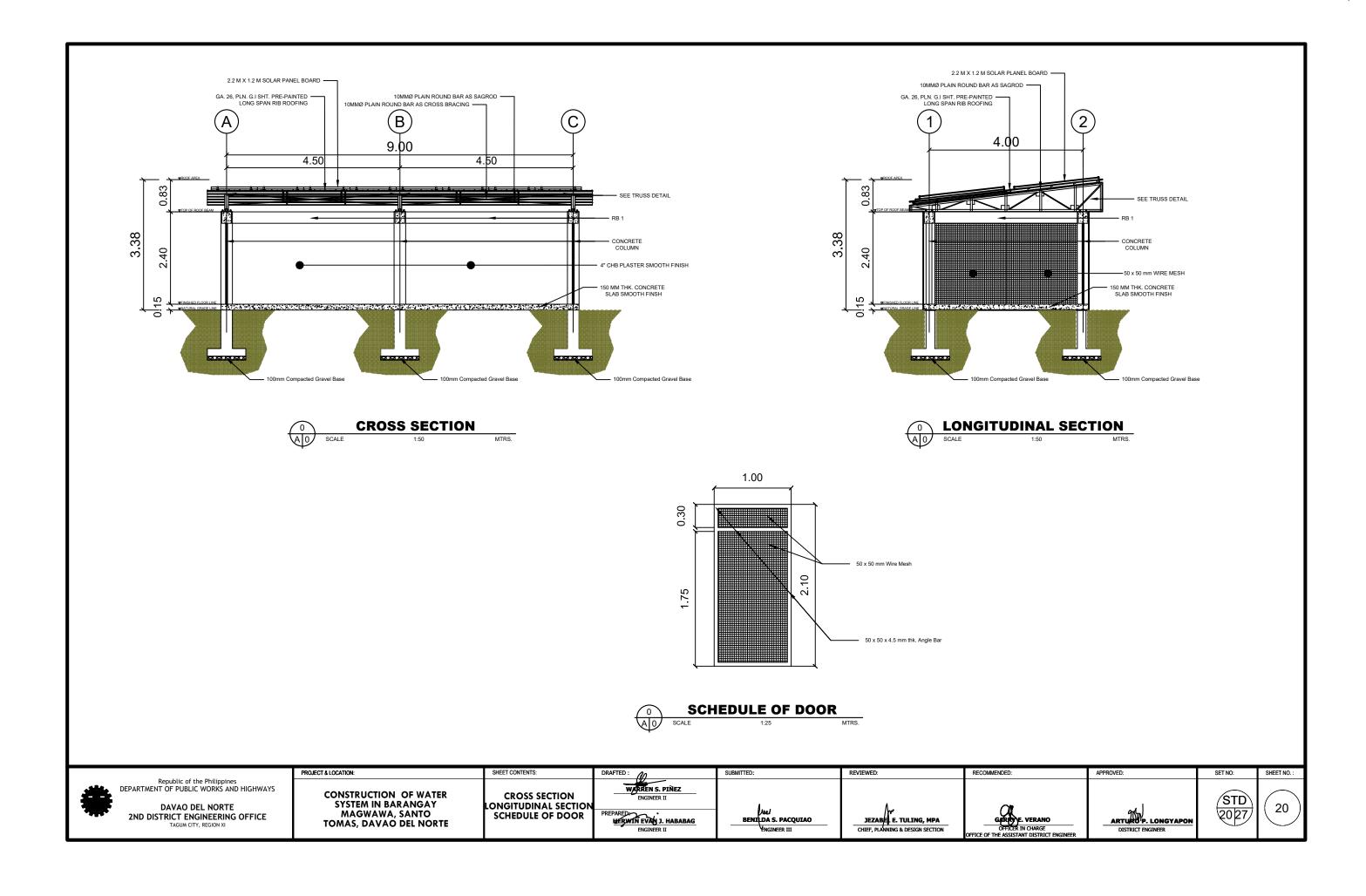
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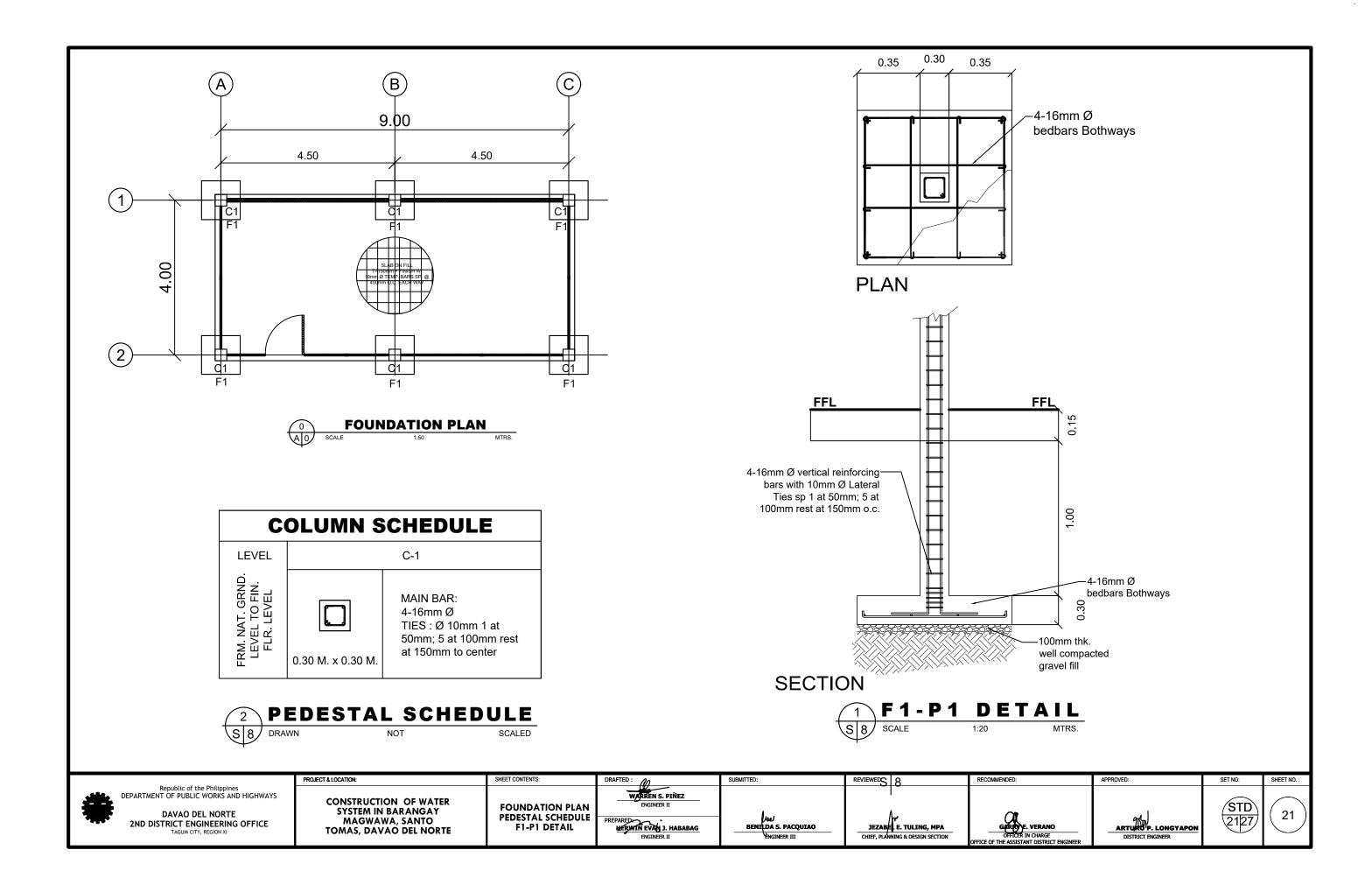


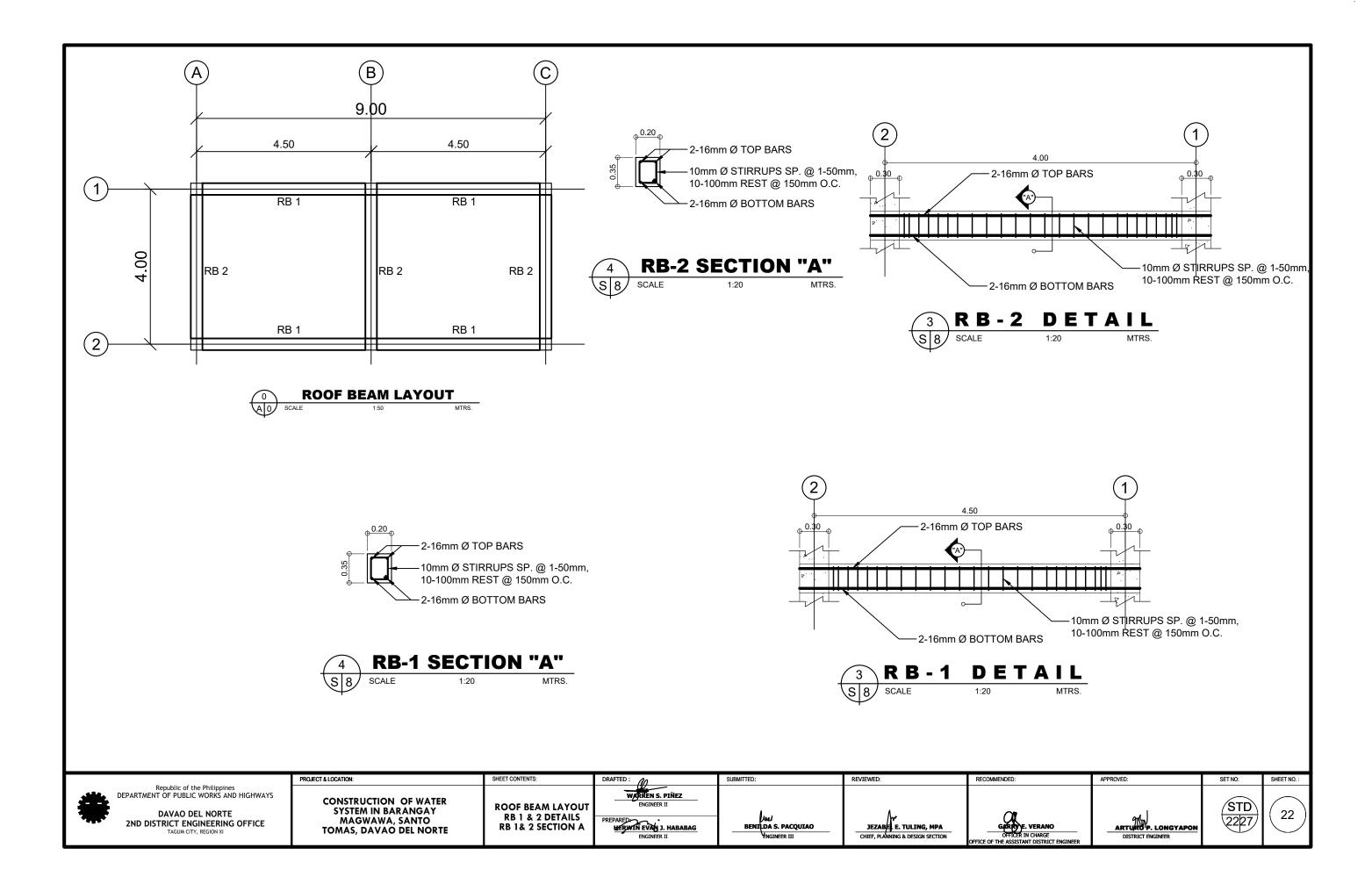
PROJECT & LOCATION: SHEET CONTENTS: DRAFTED : Republic of the Philippines	SUBMITTED:	REVIEWED:	RECOMMENDED:
Republic of the Philippines			
DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS CONSTRUCTION OF WATER			
DAVAO DEL NORTE SYSTEM IN BARANGAY PUMP HOUSE 2ND DISTRICT ENGINEERING OFFICE MAGWAWA, SANTO PERSPECTIVE TOMAS, DAVAO DEL NORTE PERSPECTIVE PERSPECTIVE	N J. HABABAG BENILDA S. PACQUIAO	JEZABEL E. TULING, MPA CHIEF, PLANNING & DESIGN SECTION	

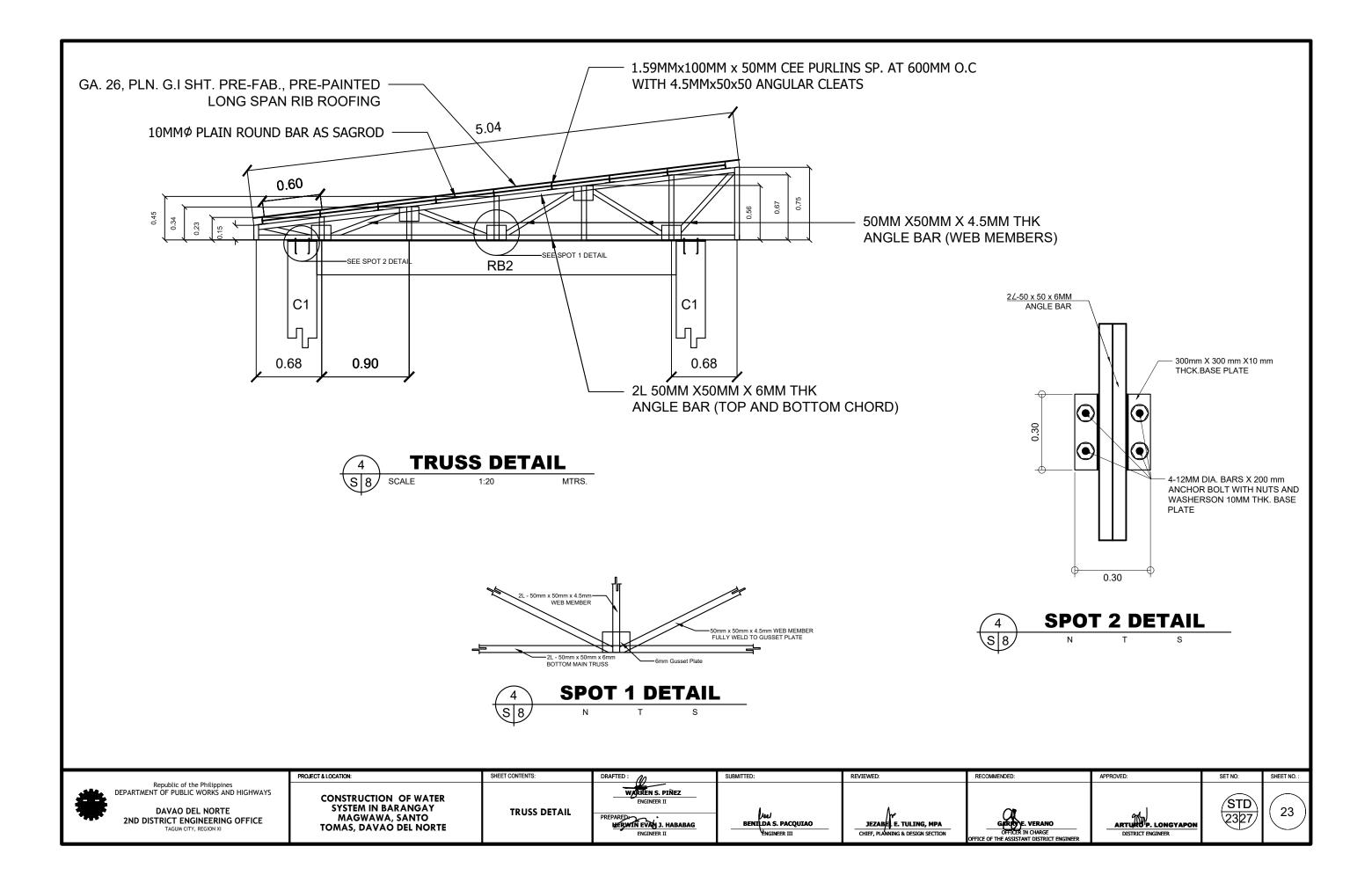


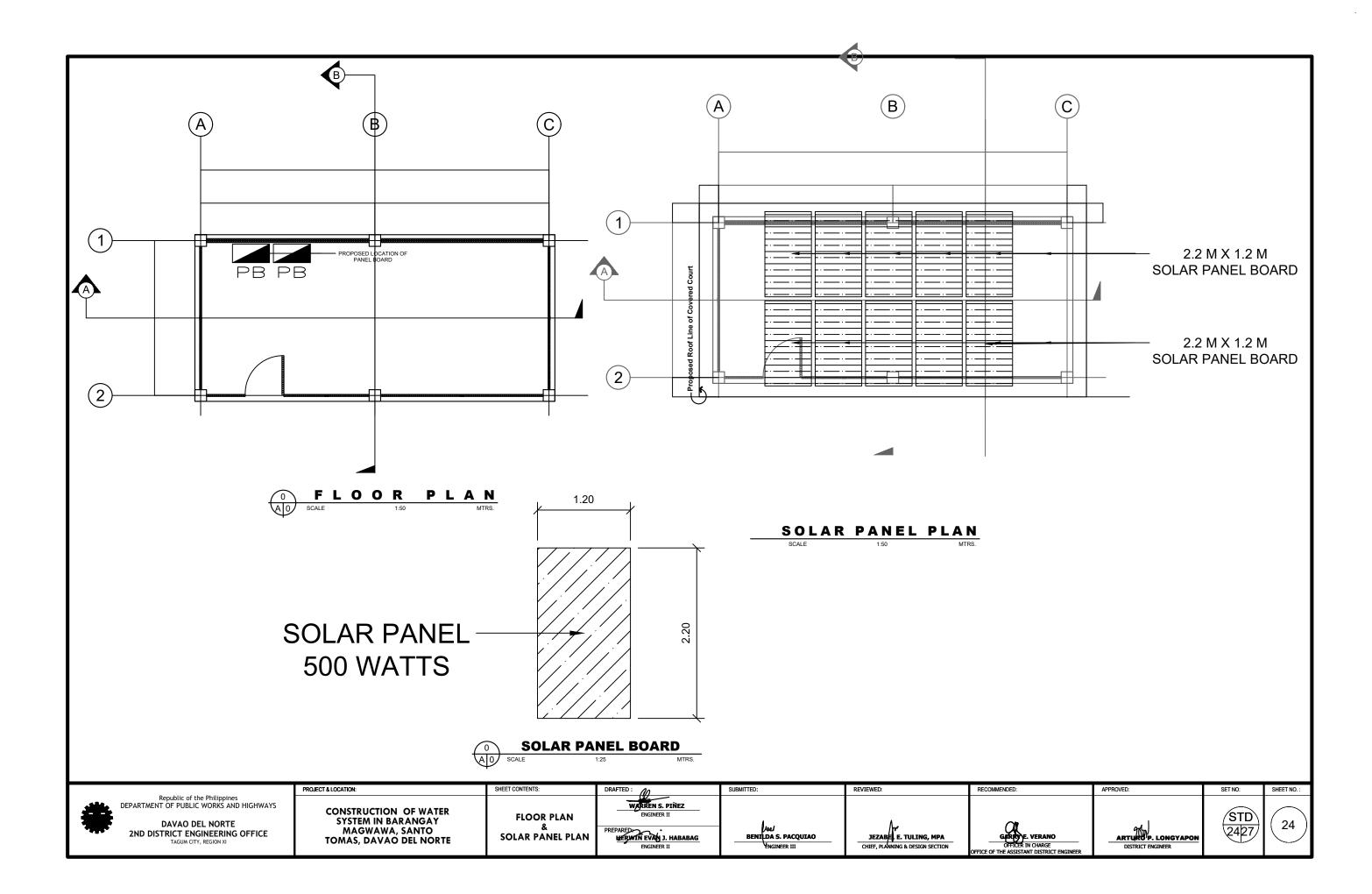


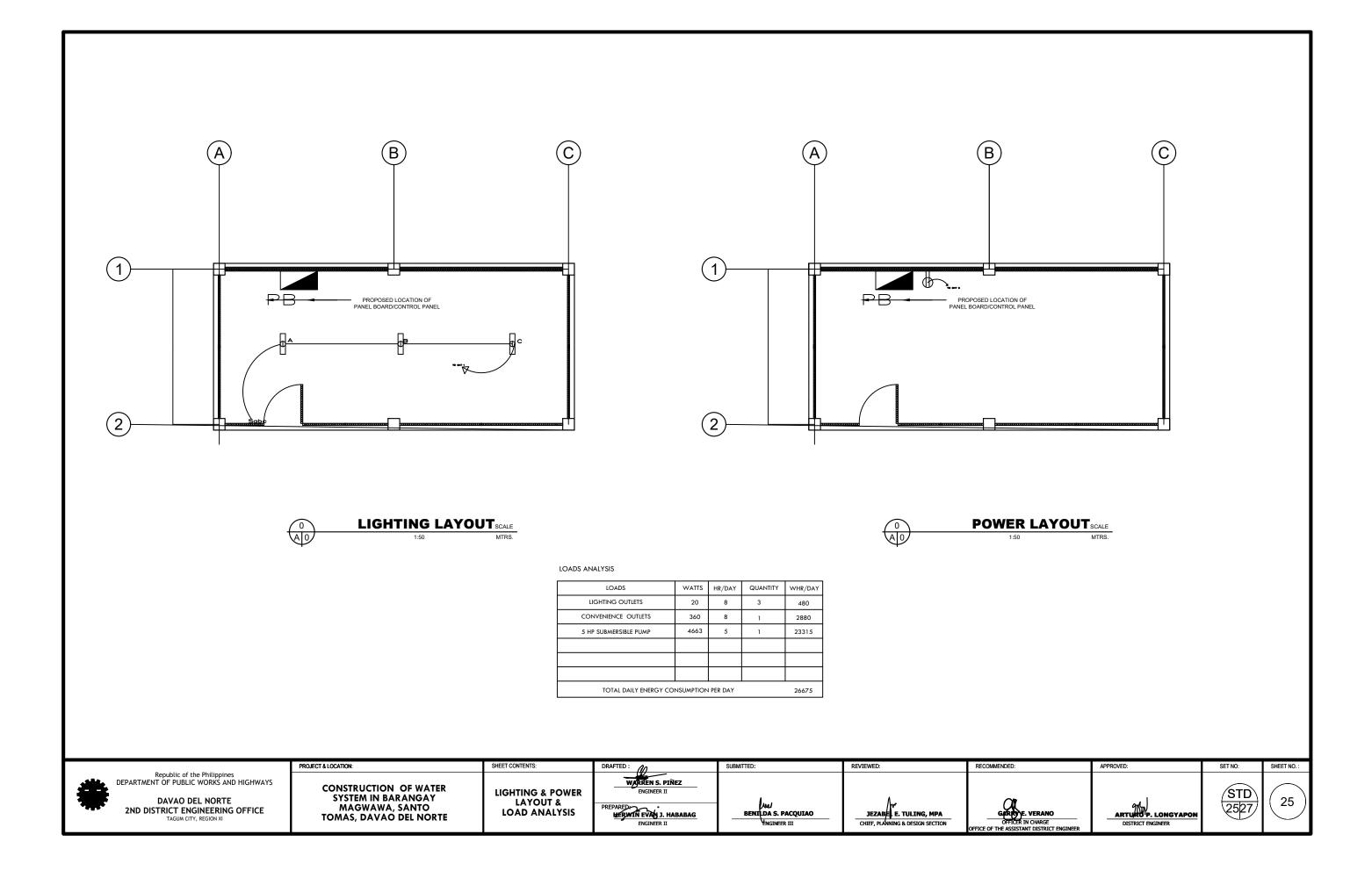


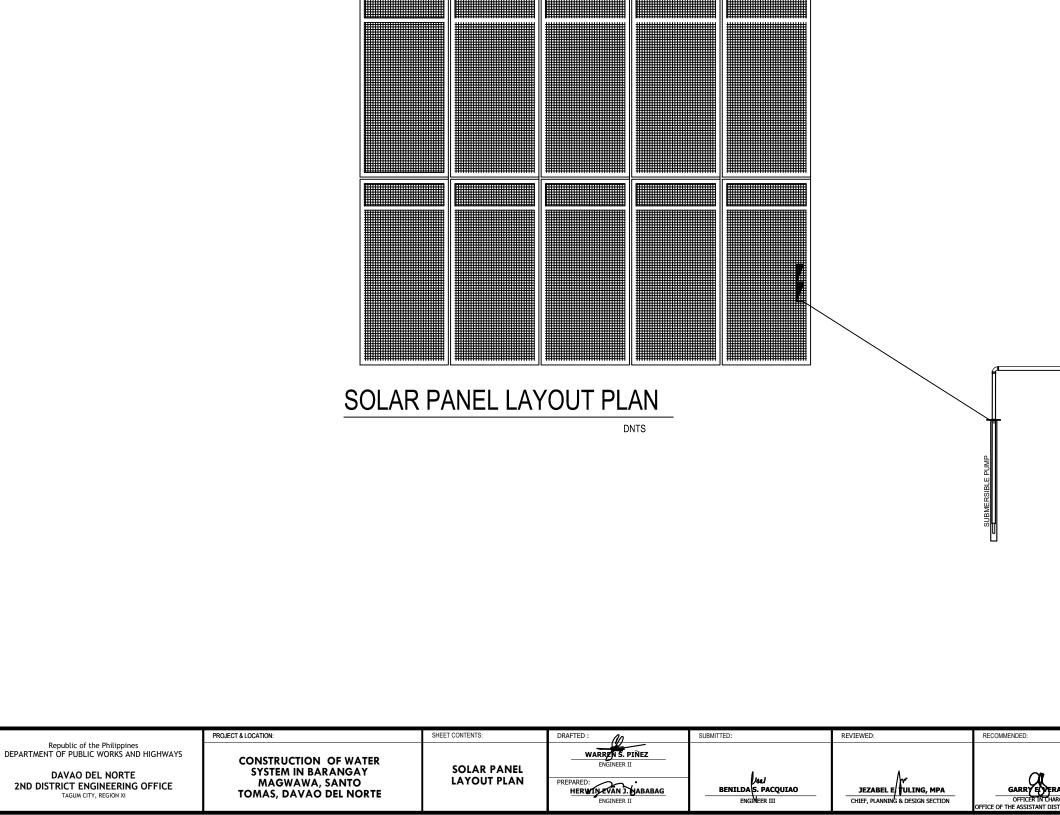






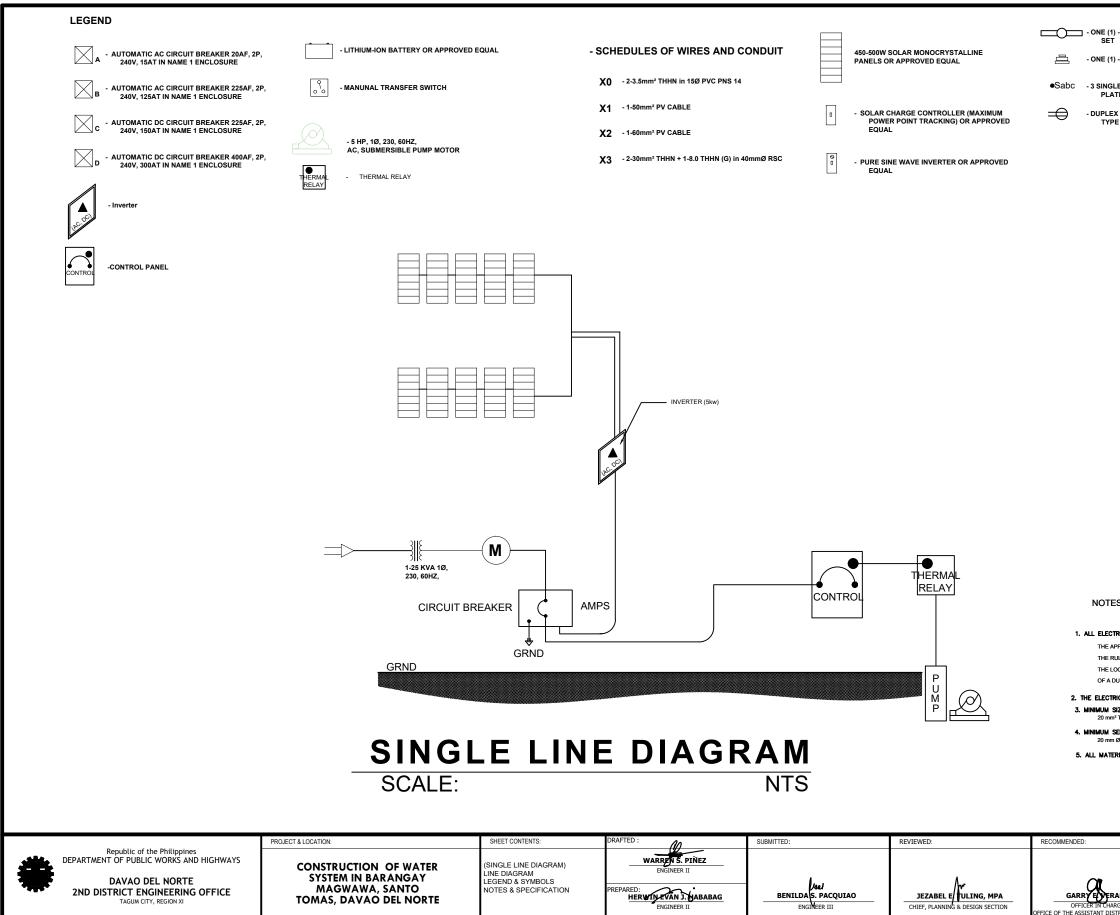






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TO ELEVATED TANK



- ONE (1) -18W 230V, T5 LED TUBE & BOX TYPE SET

- ONE (1) -20W 230V, LED FLOOD LIGHT

•Sabc - 3 SINGLE POLE WALL SWITCHES ON ONE PLATE (10A, 250V)

> - DUPLEX CONVENIENCE OUTLET, GROUNDING TYPE (20A, 250V)

NOTES & SPECIFICATIONS

1. ALL ELECTRICAL WORKS SHALL COMPLY IN ACCORDANCE WITH THESE PLANS AND SPECIFICATIONS. THE APPLICABLE PROVISIONS OF THE LATEST EDITION OF THE PHILIPPINE ELECTRICAL CODE (PEC). THE RULES AND REGULATION OF THE LOCAL ENFORCING AUTHORITY AND THE REQUIREMENTS OF THE LOCAL POWER COMPANY. THE ELECTRICAL WORKS SHALL BE UNDER IMMEDIATE SUPERVISION OF A DULY REGISTERED ELECTRICAL ENGINEER.

2. THE ELECTRICAL SERVICE POWER IS 1-PHASE, 2-WIRE, 230 V AC, 60 Hz. 3. MINIMUM SIZE OF WIRE & ELECTRICAL CONDUIT TO BE USE SHALL BE 20 mm⁻ TW & 15 mm Ø RESPECTIVELY.

4. MINIMUM SERVICE ENTRANCE SHALL BE 8.0 mm⁶ THW COPPER WIRE & 20 mm Ø RIGID STEEL CONDUIT.

5. ALL MATERIALS TO BE USED SHALL BE NEW & OF APPROVED TYPE.

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